Campus: UCLA Building Name: Easton Stadium CAAN ID: 4303 Auxiliary Building ID: 4303.3



Date: Apr 14, 2021

FORM 1 CERTIFICATE OF SEISMIC PERFORMANCE LEVEL UC-Designed & Constructed Facility

OF

UNIVERSITY

CALIFORNIA

Campus-Acquired or Leased Facility

BUILDING DATA

Building Name: Easton Stadium - Batting Cages Address: 100 De Neve Dr. Los Angeles, CA 90024 Site location coordinates: Latitude 34.07389225 Longitudinal -118.4424168

UCOP SEISMIC PERFORMANCE LEVEL (OR "RATING"): V

ASCE 41-17 Model Building Type:

- a. Longitudinal Direction: S3 (Light Metal Building)
- b. Transverse Direction: S3 (Light Metal Building)

Gross Square Footage: 1,500 Number of stories *above* grade: 1 Number of basement stories *below* grade: 0

Year Original Building was Constructed: Unknown Original Building Design Code & Year: Unknown Retrofit Building Design Code & Code (if applicable): N/A

SITE INFORMATION

| Site Class: D | Basis: | Inferred |
|-------------------|--------|---|
| Geologic Hazards: | | |
| Fault Rupture: No | Basis: | CGS Earthquake Hazards Zone Application |
| Liquefaction: Yes | Basis: | CGS Earthquake Hazards Zone Application |
| Landslide: Yes | Basis: | CGS Earthquake Hazards Zone Application |

ATTACHMENT

Original Structural Drawings: (N/A, N/A, N/A, N/A) or Seismic Evaluation: (Easton Stadium - Batting Cages Seismic Evaluation, KPFF, 4/14/2021, FEMA 154 Rapid Visual Screening) Retrofit Structural Drawings: (N/A, N/A, N/A, N/A)



CERTIFICATION & PRESUMPTIVE RATING VERIFICATION STATEMENT

I, Mark Hershberg, a California-licensed structural engineer, am responsible for the completion of this certificate, and I have no ownership interest in the property identified above. My scope of review to support the completion of this certificate included both of the following ("No" responses must include an explanation):

OF

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- a) the review of structural drawings indicating that they are as-built or record drawings, or that they otherwise are the basis for the construction of the building: □ Yes ☑ No
- b) visiting the building to verify the observable existing conditions are reasonably consistent with those shown on the structural drawings: ☑ Yes □ No

No as-built drawings were available, so evaluation performed using FEMA 154 Level 2 Rapid Visual Screening protocol on visual observations only.

Based on my review, I have verified that the UCOP Seismic Performance Level (SPL) is presumptively permitted by the following UC Seismic Program Guidebook provision (choose one of the following):

□ 1) Contract documents indicate that the original design and construction of the aforementioned building is in accordance with the benchmark design code year (or later) building code seismic design provisions for UBC or IBC listed in Table 1 below.

☑ 2) The existing SPL rating is based on an acceptable basis of seismic evaluation completed in 2006 or later.

□ 3) Contract documents indicate that a comprehensive¹ building seismic retrofit design was fullyconstructed with an engineered design based on the 1997 UBC/1998 *or later* CBC, and (choose one of the following):

□ the retrofit project was completed by the UC campus. Further, the design was based on ground motion parameters, at a minimum, corresponding to BSE-1E (or BSE-R) and BSE-2E (or BSE-C) as defined in ASCE 41, or the full design basis ground motion required in the 1997 UBC/1998 CBC *or later* for EXISTING buildings, and is presumptively assigned an SPL rating of IV.

□ the retrofit project was completed by the UC campus. Further, the design was based on ground motion parameters, at a minimum, corresponding to BSE-1 (or BSE-1N) and BSE-2 (or BSE-2N) as defined in ASCE 41, or the full design basis ground motion required in the 1997 UBC/1998 *or later* CBC for NEW buildings, and is presumptively assigned an SPL rating of III.

□ the retrofit project was not completed by the UC campus following UC policies, and is presumptively assigned an SPL rating of IV.

¹ A comprehensive retrofit addresses the entire building structural system as indicated by the associated seismic evaluation, as opposed to addressing selective portions of the structural system.

Campus: UCLA Building Name: Easton Stadium CAAN ID: 4303 Auxiliary Building ID: 4303.3



Date: Apr 14, 2021

CERTIFICATION SIGNATURE

Mark Hershberg Print Name Principal Title



6/30/2021 License Expiration Date 4/14/2021

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CALIFORNIA

OF

Date

AFFIX SEAL HERE



KPFF Inc., (213) 418-0201, 700 S. Flower St., Suite 2100, Los Angeles, CA 90017

Firm Name, Phone Number, and Address

Campus: UCLA Building Name: Easton Stadium CAAN ID: 4303 Auxiliary Building ID: 4303.3



UNIVERSITY OF CALIFORNIA

Table 1: Benchmark Building Codes and Standards

| | Building Seismic Design Provisions | | | | | |
|---|------------------------------------|------|--|--|--|--|
| Building Type ^{a,b} | UBC | IBC | | | | |
| Wood frame, wood shear panels (Types W1 and W2) | 1976 | 2000 | | | | |
| Wood frame, wood shear panels (Type W1a) | 1976 | 2000 | | | | |
| Steel moment-resisting frame (Types S1 and S1a) | 1997 | 2000 | | | | |
| Steel concentrically braced frame (Types S2 and S2a) | 1997 | 2000 | | | | |
| Steel eccentrically braced frame (Types S2 and S2a) | 1988 ^g | 2000 | | | | |
| Buckling-restrained braced frame (Types S2 and S2a) | f | 2006 | | | | |
| Metal building frames (Type S3) | f | 2000 | | | | |
| Steel frame with concrete shear walls (Type S4) | 1994 | 2000 | | | | |
| Steel frame with URM infill (Types S5 and S5a) | f | 2000 | | | | |
| Steel plate shear wall (Type S6) | f | 2006 | | | | |
| Cold-formed steel light-frame construction—shear wall system (Type CFS1) | 1997 ^h | 2000 | | | | |
| Cold-formed steel light-frame construction—strap-braced wall system (Type CFS2) | f | 2003 | | | | |
| Reinforced concrete moment-resisting frame (Type C1) ⁱ | 1994 | 2000 | | | | |
| Reinforced concrete shear walls (Types C2 and C2a) | 1994 | 2000 | | | | |
| Concrete frame with URM infill (Types C3 and C3a) | f | f | | | | |
| Tilt-up concrete (Types PC1 and PC1a) | 1997 | 2000 | | | | |
| Precast concrete frame (Types PC2 and PC2a) | f | 2000 | | | | |
| Reinforced masonry (Type RM1) | 1997 | 2000 | | | | |
| Reinforced masonry (Type RM2) | 1994 | 2000 | | | | |
| Unreinforced masonry (Type URM) | f | f | | | | |
| Unreinforced masonry (Type URMa) | f | f | | | | |
| Seismic isolation or passive dissipation | 1991 | 2000 | | | | |

Note: This table has been adapted from ASCE 41-17 Table 3-2. Benchmark Building Codes and Standards for Life Safety Structural Performed at BSE-1E. Note: UBC = Uniform Building Code. IBC = International Building Code.

^a Building type refers to one of the common building types defined in Table 3-1 of ASCE 41-17.

^b Buildings on hillside sites shall not be considered Benchmark Buildings.

^c not used

^d not used

^e not used

^f No benchmark year; buildings shall be evaluated in accordance with Section III.J.

^g Steel eccentrically braced frames with links adjacent to columns shall comply with the 1994 UBC Emergency Provisions, published September/October 1994, or subsequent requirements.

^h Cold-formed steel shear walls with wood structural panels only.

ⁱ Flat slab concrete moment frames shall not be considered Benchmark Buildings.



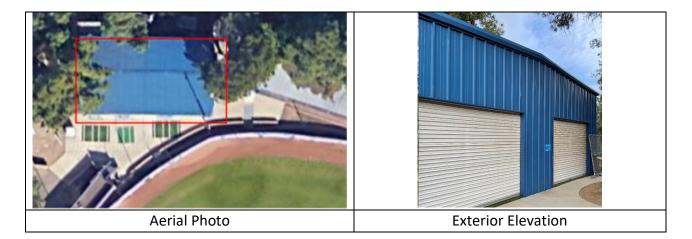
UCLA – Easton Stadium (Batting Cages)

DATE: 4/14/2021 FEMA 154 Rapid Visual Screening Minimum Building Report Information

BUILDING DATA

S 5078 PROFESSION RERSHIELD SOTE STAN S 5078 PT C CAUPON C

Campus: UCLA Building Name: Easton Stadium - Batting Cages CAAN ID: 4303 Auxiliary Building ID: 4303.3 Address: 100 De Neve Dr. Los Angeles, CA 90024 Site location coordinates: Latitude 34.07389225 Longitudinal -118.4424168



ASCE 41-17 Model Building Type:

- a. Longitudinal Direction: S3 (Light Metal Building)
- b. Transverse Direction: S3 (Light Metal Building)

Site-specific Ground Motion Study? No

Seismic Design Acceleration Parameters of Interest:

- a. For BSE-1E Sxs=0.895g and Sx1=0.515g
- b. For BSE-2E S_{XS} =1.53g and S_{X1} =0.941g

Estimated Fundamental Period (seconds)

- a. Longitudinal: Unknown
- b. Transverse: Unknown

Gross Square Footage: 1,500 Number of stories *above* grade: 1 Number of basement stories below grade: 0

Year Original Building was Constructed: Unknown Original Building Design Code & Year: Unknown Retrofit Building Design Code & Code (if applicable): N/A

SITE INFORMATION

| Site Class: D | Basis: | Inferred |
|-------------------|--------|---|
| Geologic Hazards: | | |
| Fault Rupture: No | Basis: | CGS Earthquake Hazards Zone Application |
| Liquefaction: Yes | Basis: | CGS Earthquake Hazards Zone Application |
| Landslide: Yes | Basis: | CGS Earthquake Hazards Zone Application |

UCOP SEISMIC PERFORMANCE RATING (OR "RATING"): V

"BALLPARK" RETROFIT COST (if applicable)

- \boxtimes Minor (<\$50/sf)
- □ Moderate (~\$50-\$200/sf)
- □ Major (>\$200/sf)

SUMMARY TIER 1 SEISMIC EVALUATION STRUCTURAL NON-COMPLIANCES/FINDINGS SIGNIFICANTLY AFFECTING RATING DETERMINATION

Significant Structural Deficiencies, Potentially Affecting Seismic Performance Level Designation:

- \boxtimes Lateral System Stress Check (wall shear, column shear or flexure, or brace axial as applicable)
- Lateral System Detailing (reinforcement ratio, confinement, aspect ratio, etc)
- Load Path
- **Adjacent Buildings**
- Weak Story
- Soft Story
- Geometry (vertical irregularities)
- Torsion
- Mass Vertical Irregularity
- **Cripple Walls**
- Wood Sills (bolting)
- Diaphragm Continuity
- Openings at Shear Walls (concrete or masonry)
- \boxtimes Liquefaction

- Slope Failure
- Surface Fault Rupture
- Masonry or Concrete Wall Anchorage at Diaphragm
- URM wall height to thickness ratio
- **URM Parapets or Cornices**
- **URM Chimney**
- Heavy Partitions Braced by Ceilings
- Appendages

POTENTIAL FALLING HAZARDS

- Heavy ceilings, features or ornamentation above large lecture halls, auditoriums, lobbies or other areas where large numbers of people congregate.
- Heavy masonry or stone veneer above exit ways.
- Unbraced masonry parapets, cornices or other ornamentation above exit ways.
- Unrestrained hazardous materials storage.
- Masonry chimneys.
- Unrestrained natural gas-fueled equipment such as water heaters, boilers, emergency generators, etc.
- \square None of the above.

BRIEF DESCRIPTION OF ANTICIPATED FAILURE MECHANISM

COMMENTS AND RECOMMENDATIONS

A FEMA 154 Level 2 Rapid Visual Screening was performed in lieu of an ASCE Tier 1 evaluation due to construction type and lack of as-built documentation. Lack of redundancy is the most significant factor in the rating determination.

Appendices

A. FEMA 154 Rapid Visual Screening

Rapid Visual Screening of Buildings for Potential Seismic Hazards FEMA P-154 Data Collection Form

Level 1 VERY HIGH Seismicity

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| FEMA BUILDING TYPE Do Not | W1 | W1A | W2 | S1 | S2 | S 3 | S4 | S5 | C1 | C2 | C3 | PC1 | PC2 | RM1 | RM2 | URM | MH |
| Know | | | | (MRF) | (BR) | (LM) | (RC SW) | (URM INF) | (MRF) | (SW) | (URM INF) | (TU) | | (FD) | (RD) | | |
| Basic Score | 2.1 | 1.9 | 1.8 | 1.5 | 1.4 | 1.6 | 1.4 | 1.2 | 1.0 | 1.2 | 0.9 | 1.1 | 1.0 | 1.1 | 1.1 | 0.9 | 1.1 |
| Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} | -0.9 -0.6 | -0.9 -0.5 | -0.9 -0.5 | -0.8 -0.4 | -0.7 -0.4 | -0.8 -0.5 | -0.7 -0.4 | -0.7 -0.3 | -0.7 -0.4 | -0.8 -0.4 | -0.6 -0.3 | -0.7 -0.4 | -0.7 -0.4 | -0.7 -0.4 | -0.7 -0.4 | -0.6 -0.3 | NA NA |
| Plan Irregularity, P_{L1} | -0.7 | -0.7 | -0.6 | -0.5 | -0.5 | -0.6 | -0.4 | -0.4 | -0.4 | -0.5 | -0.3 | -0.5 | -0.4 | -0.4 | -0.4 | -0.3 | NA |
| Pre-Code | -0.3 | -0.3 | -0.3 | -0.3 | -0.2 | -0.3 | -0.2 | -0.1 | -0.1 | -0.2 | 0.0 | -0.2 | -0.1 | -0.2 | -0.2 | 0.0 | 0.0 |
| Post-Benchmark | 1.9 | 1.9 | 2.0 | 1.0 | 1.1 | 1.1 | 1.5 | NA | 1.4 | 1.7 | NA | 1.5 | 1.7 | 1.6 | 1.6 | NA | 0.5 |
| Soil Type A or B | 0.5 | 0.5 | 0.4 | 0.3 | 0.3 | 0.4 | 0.3 | 0.2 | 0.2 | 0.3 | 0.1 | 0.3 | 0.2 | 0.3 | 0.3 | 0.1 | 0.1 |
| Soil Type E (1-3 stories) Soil Type E (> 3 stories) | 0.0 -0.4 | -0.2 -0.4 | -0.4 -0.4 | -0.3 -0.3 | -0.2 -0.3 | -0.2 NA | -0.2 -0.3 | -0.1 -0.1 | -0.1 -0.1 | -0.2 -0.3 | 0.0 -0.1 | -0.2 NA | -0.1 -0.1 | -0.2 -0.2 | -0.2 -0.2 | 0.0 0.0 | -0.1 NA |
| Minimum Score, S _{MIN} | 0.7 | 0.7 | 0.7 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.3 | 0.3 | 0.3 | 0.2 | 0.2 | 0.3 | 0.3 | 0.2 | 1.0 |
| FINAL LEVEL 1 SCORE, $S_{L1} \ge S_{MIN}$: | | | | 1 | | | | | | | | | | | | | 1.6 |
| EXTENT OF REVIEW | | | | OTHEF | R HAZ | | ; | | ACT | ION R | EQUIF | RED | | | | | |
| Exterior: 🗌 Partial | All Sides | 🗌 Aer | al | Are There | | | | ۱ | | ed Struc | | | Require | ed? | | | |
| Interior: 🗌 None | Visible | Ent | ered | Detailed | | | | | | es, unkno | | | • | | uilding | | |
| Drawings Reviewed: 🗌 Yes | N I - | | | Poun | ding pot | · · | nless SL2 | > | | es, score es, other | | | | | - | | |
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| Soil Type Source: Geologic Hazards Source: CGS Earth | | azards | App. | cut-o | ff, if knov g hazaro | | aller adja | cent | | | 1020103 | | | | | | |
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Rapid Visual Screening of Buildings for Potential Seismic Hazards

Level 2 (Optional)

FEMA P-154 Data Collection Form Optional Level 2 data collection to be performed by a civil or structural engineering professional, architect, or graduate student with background in seismic evaluation or design of buildings.

| Bldg Name: Batting Cages | Final Level 1 Score: | $S_{L1} = 1.6$ | (do not consider S_{MIN}) |
|--------------------------|---------------------------------|---|---------------------------------|
| Screener: | Level 1 Irregularity Modifiers: | Vertical Irregularity, $V_{L1} = 0$ | Plan Irregularity, $P_{L1} = 0$ |
| Date/Time: | ADJUSTED BASELINE SCORE: | $S' = (S_{L1} - V_{L1} - P_{L1}) = 1.6$ | |

| Торіс | Statement (| f statement is true, circle the "Yes" modifier; oth | herwise cross out the modifier.) | Yes | Subtotals | | | |
|--|-------------------------|---|--|---------------|--|--|--|--|
| Vertical | Sloping | W1 building: There is at least a full story grade | e change from one side of the building to the other. | -0.9/ | | | | |
| rregularity, VL2 | Site | Non-W1 building: There is at least a full story | grade change from one side of the building to the other. | -0.2 | | | | |
| | Weak | W1 building cripple wall: An unbraced cripple | | -Ø.5 | | | | |
| | and/or | W1 house over garage: Underneath an occup | pied story, there is a garage opening without a steel moment frame, | 1 | | | | |
| | Soft Story | and there is less than 8' of wall on the same lin | ine (for multiple occupied floors above, use 16' of wall minimum). | -0.9/ | | | | |
| | (circle one maximum) | W1A building open front: There are openings length of the building. | s at the ground story (such as for parking) over at least 50% of the | -0/9 | | | | |
| | | | any story is less than 50% of that at story above or height of any story above. | 0.7 | | | | |
| | | | any story is between 50% and 75% of that at story above or height | -0.4 | | | | |
| | Setback | Vertical elements of the lateral system at an u diaphragm to cantilever at the offset. | pper story are outboard of those at the story below causing the | -0.7 | | | | |
| | | | er stories are inboard of those at lower stories. | -0.4 | | | | |
| | | | ents that is greater than the length of the elements. | -0.2 | | | | |
| | Short Column/ | C1,C2,C3,PC1,PC2,RM1,RM2: At least 20% of height/depth ratios less than 50% of the nomin | of columns (or piers) along a column line in the lateral system have inal height/depth ratio at that level. | -0.4 | | | | |
| | Pier | C1,C2,C3,PC1,PC2,RM1,RM2: The column d or there are infill walls or adjacent floors that s | depth (or pier width) is less than one half of the depth of the spandrel, shorten the column. | -0.4 | | | | |
| | Split Level | There is a split level at one of the floor levels of | or at the roof. | -Ø.4 | 0 | | | |
| | Other | There is another observable severe vertical irr | regularity that obviously affects the building's seismic performance. | - þ .7 | $V_{L2} = 0$ | | | |
| | Irregularity | There is another observable moderate vertical | al irregularity that may affect the building's seismic performance. | 10.4 | (Cap at -0.9 | | | |
| Plan Irregularity, <i>P</i> _{L2} | | gularity: Lateral system does not appear relative /1A open front irregularity listed above.) | rely well distributed in plan in either or both directions. (Do not | -0,5 | | | | |
| | | | lements of the lateral system that are not orthogonal to each other. | -0.2 | | | | |
| | Reentrant co | entrant corner: Both projections from an interior corner exceed 25% of the overall plan dimension in that direction. | | | | | | |
| | | Diaphragm opening: There is an opening in the diaphragm with a width over 50% of the total diaphragm width at that level. | | | | | | |
| | C1, C2 build | C1, C2 building out-of-plane offset: The exterior beams do not align with the columns in plan. | | | | | | |
| | Other irregul | arity: There is another observable plan irregulari | rity that obviously affects the building's seismic performance. | -0.5 | P _{L2} = <u>0</u> (Cap at -0.7 | | | |
| Redundancy | The building | nas at least two bays of lateral elements on eac | | +0.2 | | | | |
| Pounding Building is | | | oors do not align vertically within 2 feet. (Cap total | -0.7 | | | | |
| | | 1.5% of the height of the shorter of One be | puilding is 2 or more stories taller than the other. pounding | -0.17 | | | | |
| | the building a | nd adjacent structure and: The bu | uilding is at the end of the block. modifiers at -0.9) | -0/4 | | | | |
| S2 Building | "K" bracing g | eometry is visible. | | -Q.7 | | | | |
| C1 Building | | ves as the beam in the moment frame. | | -Ø.3 | | | | |
| PC1/RM1 Bldg | | f-to-wall ties that are visible or known from draw ark or retrofit modifier.) | wings that do not rely on cross-grain bending. (Do not combine with | -0.2 | | | | |
| PC1/RM1 Bldg | The building | nas closely spaced, full height interior walls (rat | ther than an interior space with few walls such as in a warehouse). | +0.2 | | | | |
| JRM | Gable walls a | | | -0.3 | | | | |
| MH | There is a su | pplemental seismic bracing system provided be | etween the carriage and the ground. | +0.5 | • | | | |
| Retrofit | | ve seismic retrofit is visible or known from draw | | +1.2 | м=_0 | | | |
| | | | 1.6 + 0 = 1.6 | | to Level 1 fori | | | |
| | | | | TUINIE | | | | |
| nere is odserva | | leterioration or another condition that negatively | ly affects the building's seismic performance: | | | | | |

| Location | Statement (Check "Yes" or "No") | Yes | No | Comment |
|--------------|---|------------|-----------------|---------|
| Exterior | There is an unbraced unreinforced masonry parapet or unbraced unreinforced masonry chimney. | | | |
| | There is heavy cladding or heavy veneer. | | | |
| | There is a heavy canopy over exit doors or pedestrian walkways that appears inadequately supported. | | | |
| | There is an unreinforced masonry appendage over exit doors or pedestrian walkways. | | | |
| | There is a sign posted on the building that indicates hazardous materials are present. | | | |
| | There is a taller adjacent building with an unanchored URM wall or unbraced URM parapet or chimney. | | | |
| | Other observed exterior nonstructural falling hazard: | | | |
| Interior | There are hollow clay tile or brick partitions at any stair or exit corridor. | | / | |
| | Other observed interior nonstructural falling hazard: | | / | |
| Estimated No | nstructural Seismic Performance (Check appropriate box and transfer to Level 1 form conclusions) | | | |
| | □ Potential nonstructural hazards with significant threat to occupant life safety →Detailed Nonstructur | ral Evalua | ation recomme | ended |
| | ☐ Nonstructural hazards identified with significant threat to occupant life safety →But no Detailed Nor | nstructura | al Evaluation r | equired |
| | ✓ Low or no nonstructural hazard threat to occupant life safety →No Detailed Nonstructural Evaluation | on require | ed | |

Comments: