Building Name: Building C (Press Box)

CAAN ID: 4220B

Auxiliary Building ID: 4220B.1



FORM 1 CERTIFICATE OF SEISMIC PERFORMANCE LEVEL

UC-Designed & Constructed Facility

☐ Campus-Acquired or Leased Facility

BUILDING DATA

Building Name: Building C (Press Box)

Address: 100 Constitution Ave, Los Angeles, CA, 90095

Site location coordinates: Latitude 34.0593801 Longitudinal -118.4592182

UCOP SEISMIC PERFORMANCE LEVEL (OR "RATING"): ∨

ASCE 41-17 Model Building Type:

a. Longitudinal Direction: RM1: Reinforced Masonry Walls w/ Flexible Diaphragms

b. Transverse Direction: RM1: Reinforced Masonry Walls w/ Flexible Diaphragms

Gross Square Footage: 440 Number of stories *above* grade: 1

Number of basement stories below grade: 0

Year Original Building was Constructed: 1984 Original Building Design Code & Year: UBC-1979

Retrofit Building Design Code & Code (if applicable): N/A

SITE INFORMATION

Site Class: D Basis: Inferred

Geologic Hazards:

Fault Rupture: No Basis: Inferred Liquefaction: No Basis: Inferred Landslide: No Basis: Inferred

ATTACHMENT

Original Structural Drawings: (UCLA Renovation of Sawtelle Field Phase 2, Gerald Lehmer Associates, 6/1/1982)

Seismic Evaluation: (Building A Seismic Evaluation Tier 1, KPFF, 01/29/2020, ASCE 41-17 Tier 1)

Retrofit Structural Drawings: (N/A)

Date: Jan 29, 2021

Building Name: Building C (Press Box)

CAAN ID: 4220B





Date: Jan 29, 2021

CERTIFICATION & PRESUMPTIVE RATING VERIFICATION STATEMENT

I, Mark Hershberg, a California-licensed structural engineer, am responsible for the completion of this certificate, and I have no ownership interest in the property identified above. My scope of review to support the completion of this certificate included both of the following ("No" responses must include an explanation):

 a) the review of structural drawings indicating that otherwise are the basis for the construction of t b) visiting the building to verify the observable exist those shown on the structural drawings: □ Yes Due to COVID-19 protocols, observations were p 	he building: ☑ Yes □ No sting conditions are reasonably consistent with ☑ No
Based on my review, I have verified that the UCOP Sei permitted by the following UC Seismic Program Guide	
\Box 1) Contract documents indicate that the original debuilding is in accordance with the benchmark design contractions for UBC or IBC listed in Table 1 below.	_
lackiv 2) The existing SPL rating is based on an acceptable later.	basis of seismic evaluation completed in 2006 or
 3) Contract documents indicate that a comprehens constructed with an engineered design based on the 1 the following): 	-
□ the retrofit project was completed by the UC can motion parameters, at a minimum, corresponding defined in ASCE 41, or the full design basis ground later for EXISTING buildings, and is presumptively and the retrofit project was completed by the UC can motion parameters, at a minimum, corresponding defined in ASCE 41, or the full design basis ground CBC for NEW buildings, and is presumptively assign the retrofit project was not completed by the Up presumptively assigned an SPL rating of IV.	to BSE-1E (or BSE-R) and BSE-2E (or BSE-C) as motion required in the 1997 UBC/1998 CBC or assigned an SPL rating of IV. mpus. Further, the design was based on ground to BSE-1 (or BSE-1N) and BSE-2 (or BSE-2N) as motion required in the 1997 UBC/1998 or later and SPL rating of III.

¹ A comprehensive retrofit addresses the entire building structural system as indicated by the associated seismic evaluation, as opposed to addressing selective portions of the structural system.

Building Name: Building C (Press Box)

CAAN ID: 4220B

Signature

Angeles, CA 90017

Auxiliary Building ID: 4220B.1



Date: Jan 29, 2021

CERTIFICATION SIGNATURE

Mark Hershberg
Print Name

Title

S5078

CA Professional Registration No.

Date

01/29/2021

KPFF Inc., (213) 418-0201, 700 S. Flower St., Suite 2100, Los

Firm Name, Phone Number, and Address



Building Name: Building C (Press Box)

CAAN ID: 4220B

Auxiliary Building ID: 4220B.1



Table 1: Benchmark Building Codes and Standards

	Building Seismic Design Provisions	
Building Type ^{a,b}	UBC	IBC
Wood frame, wood shear panels (Types W1 and W2)	1976	2000
Wood frame, wood shear panels (Type W1a)	1976	2000
Steel moment-resisting frame (Types S1 and S1a)	1997	2000
Steel concentrically braced frame (Types S2 and S2a)	1997	2000
Steel eccentrically braced frame (Types S2 and S2a)	1988 ^g	2000
Buckling-restrained braced frame (Types S2 and S2a)	f	2006
Metal building frames (Type S3)	f	2000
Steel frame with concrete shear walls (Type S4)	1994	2000
Steel frame with URM infill (Types S5 and S5a)	f	2000
Steel plate shear wall (Type S6)	f	2006
Cold-formed steel light-frame construction—shear wall system (Type CFS1)	1997 ^h	2000
Cold-formed steel light-frame construction—strap-braced wall system (Type CFS2)	f	2003
Reinforced concrete moment-resisting frame (Type C1) ⁱ	1994	2000
Reinforced concrete shear walls (Types C2 and C2a)	1994	2000
Concrete frame with URM infill (Types C3 and C3a)	f	f
Tilt-up concrete (Types PC1 and PC1a)	1997	2000
Precast concrete frame (Types PC2 and PC2a)	f	2000
Reinforced masonry (Type RM1)	1997	2000
Reinforced masonry (Type RM2)	1994	2000
Unreinforced masonry (Type URM)	f	f
Unreinforced masonry (Type URMa)	f	f
Seismic isolation or passive dissipation	1991	2000

Note: This table has been adapted from ASCE 41-17 Table 3-2. Benchmark Building Codes and Standards for Life Safety Structural Performed at BSE-1E.

Note: UBC = Uniform Building Code . IBC = International Building Code .

Date: Jan 29, 2021

 $^{^{\}rm a}\,$ Building type refers to one of the common building types defined in Table 3-1 of ASCE 41-17.

^b Buildings on hillside sites shall not be considered Benchmark Buildings.

^c not used

^d not used

e not used

 $^{^{\}it f}$ No benchmark year; buildings shall be evaluated in accordance with Section III.J.

g Steel eccentrically braced frames with links adjacent to columns shall comply with the 1994 UBC Emergency Provisions, published September/October 1994, or subsequent requirements.

 $^{^{\}it h}$ Cold-formed steel shear walls with wood structural panels only.

¹ Flat slab concrete moment frames shall not be considered Benchmark Buildings.



UCLA – Jackie Robinson Stadium, Building C (Press Box)

DATE: 1/29/2021

ASCE 41-17 Tier 1 Seismic Evaluation
Minimum Building Report Information

BUILDING DATA

Campus: UCLA

Building Name: Building C - Press Box

CAAN ID: 4220B

Auxiliary Building ID: 4220B.1

Address: 100 Constitution Ave, Los Angeles, CA, 90095

Site location coordinates: Latitude 34.0593801 Longitudinal -118.4592182







Aerial Photo

Exterior Elevation

ASCE 41-17 Model Building Type:

a. Longitudinal Direction: RM1: Reinforced Masonry Walls w/ Flexible Diaphragms

b. Transverse Direction: RM1: Reinforced Masonry Walls w/ Flexible Diaphragms

Site-specific Ground Motion Study? No

Seismic Design Acceleration Parameters of Interest:

a. For BSE-1E S_{XS} =0.892g and S_{X1} =0.512g b. For BSE-2E S_{XS} =1.829g and S_{X1} =0.937g

Estimated Fundamental Period (seconds)

a. Longitudinal: 0.11sb. Transverse: 0.11s

Gross Square Footage: 440

Number of stories above grade: 1

Number of basement stories below grade: 0

Year Original Building was Constructed: 1984 Original Building Design Code & Year: UBC-1979

Retrofit Building Design Code & Code (if applicable): N/A

SITE INFORMATION

Site Class: D Basis: Inferred

Geologic Hazards:

Fault Rupture: No Basis: Inferred Liquefaction: No Basis: Inferred Landslide: No Basis: Inferred

UCOP SEISMIC PERFORMANCE RATING (OR "RATING"): ∨

"BALLPARK" RETROFIT COST (if applicable)

■ Moderate (~\$50-\$200/sf)

■ Major (>\$200/sf)

SUMMARY TIER 1 SEISMIC EVALUATION STRUCTURAL NON-COMPLIANCES/FINDINGS SIGNIFICANTLY AFFECTING RATING DETERMINATION

Significant Structural Deficiencies, Potentially Affecting Seismic Performance Level Designation:

 · · · · · · · · · · · · · · · · · · ·	-
Lateral System Stress Check (wall shear, column shear or flexure, or brace axial a	IS
applicable)	

Lateral System Detailing (reinforcement ratio, confinement, aspect ratio, etc)

Adjacent Buildings

Weak Story

☐ Soft Story

Geometry (vertical irregularities)

Mass – Vertical Irregularity

Cripple Walls

Wood Sills (bolting)

Diaphragm Continuity

Openings at Shear Walls (concrete or masonry)

Liquefaction

	Slope Failure
	Surface Fault Rupture
\boxtimes	Masonry or Concrete Wall Anchorage at Diaphragm
	URM wall height to thickness ratio
	URM Parapets or Cornices
	URM Chimney
	Heavy Partitions Braced by Ceilings
	Appendages

POTENTIAL FALLING HAZARDS

	Heavy ceilings, features or ornamentation above large lecture halls, auditoriums
	lobbies or other areas where large numbers of people congregate.
	Heavy masonry or stone veneer above exit ways.
	Unbraced masonry parapets, cornices or other ornamentation above exit ways.
	Unrestrained hazardous materials storage.
	Masonry chimneys.
	Unrestrained natural gas-fueled equipment such as water heaters, boilers,
	emergency generators, etc.
\boxtimes	None of the above.

Due to current COVID-19 protocols, we did not verify in field that as-built documentation match current conditions or perform any condition assessment of the existing structure to identify falling hazards as required by the UCOP SSP.

BRIEF DESCRIPTION OF ANTICIPATED FAILURE MECHANISM

The diaphragm at Building C is not directly connected to the masonry shear walls and could be sensitive to load transfer issues, particularly at walls oriented in the transverse direction of the structure. The walls oriented in this direction do not have a clearly detailed load path for the transfer of inertial forces from the diaphragm to the reinforced masonry walls. It is assumed that forces are transferred via longitudinally oriented walls and headers that would have to span in their weak axis to transfer the loads. We were not able to justify these load paths bath based on the prescribed checks in Tier 1.

COMMENTS AND RECOMMENDATIONS

It is recommended that a Tier 2 evaluation be performed. A Tier 2 evaluation will provide more information related to the potential load path issues observed at Building C that were not necessarily covered in the Tier 1 evaluation .

Appendices

- A. ASCE 41-17 Tier 1 Checklists
- B. Quick Check Calculations